

# The Behaviour of Symmetric and Unsymmetric Tall Buildings Under Different Position Of Shear Wall

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## **ABSTRACT:**

A shear wall is vertical element of system which is designed to resist lateral forces due to wind and earthquake, which is one of the most efficient methods of maintaining the lateral stability of tall buildings. These walls used to absorb shear stresses and avoid construction site relocation and subsequently devastation. In case, if the shear walls are not constructed, we cannot expect the structure to exhibit acceptable tensional behavior. The contribution of the structural elements other than shear wall to the bending moment, shear force, torsion, axial force, and the final design of all structural components, are also impacted by shear wall. Over the last two decades, there has been an exponential growth in the building of towering skyscrapers above 150 meters in height. Also, Numerous identical buildings have been constructed across the Middle East and Asia, and many more are now being planned or constructed. Buildings taller than 300 meters height provide significant engineering challenges, particularly in terms of structural and geotechnical design. Wind analysis is crucial for all tall constructions. Different studies have explored the structural behavior of tall buildings with SSI by considering a range of criteria, including foundation type, soil conditions, lateral loads etc. The current study presents G+18-story rectangular building and unsymmetric building with a 3 m floor-to-floor height was evaluated in ETABS in zone III. The structure's resistance to the static and dynamic wind and seismic forces has been studied using shear walls in different locations, such as without shear walls, shear walls in the outer center, and shear walls at the corners. The results obtained after this are compared in the form of storey drift, joint displacement and base shear. The research indicates that the shear wall at outer Centre position with firm soil has the best response compared to without shear wall and shear wall at corner position for symmetrical building. And for unsymmetrical building shear wall at corner condition has best response compared to without shear wall and shear wall at outer centre condition.

**Keywords:** ETABS, Tall buildings, symmetrical, unsymmetrical, shear wall, lateral loads.

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## **I. INTRODUCTION**

The population of India is estimated to be 1,413,829,343 on January 1, 2023, making India the second most populous country in the world. In this case, the vertical growth of the built environment is inevitable to provide them with shelter and space to work. Analysis of these dynamic tall buildings with all safety factors has become a challenge for engineers. Earthquake-resistant high-rise buildings that behave well in all types of soil conditions, especially the soft soil where it is to be built. Wind analysis is very important for tall buildings.

The last two decades have seen a remarkable increase in construction of tall buildings and an almost exponential rate of growth. A significant number of these buildings have been constructed in the Middle East and Asian countries, and many more are either planned or already under construction. "Super-tall" buildings in excess of 300m in height are also presenting new challenges to engineers, particularly in relation to structural and geotechnical design. Wind analysis is one of the major factor in case of tall buildings. Many of the traditional design methods cannot be applied with any confidence as they require extrapolation well beyond the realms of prior experience, and accordingly, structural and geotechnical designers are being forced to utilize more sophisticated methods of analysis and design.

The investigations have been carried out by many researchers on the structural behavior of tall buildings with SSI by considering many parameters like foundation type, soil conditions, lateral forces, ratio of flexural stiffness of beam and column etc. Very few investigations have been carried out on soil-structure interaction of tall buildings under clayey soil conditions, which are particularly in Indian seismic zones.

There are a number of characteristics of tall buildings which can have a significant influence on foundation design, including the following:

1. Lateral loads and moments due to wind are often cyclic. Therefore, the effect of cyclic vertical and lateral

loading on the foundation system must be considered, because cyclic loading can weaken the capacity of the foundation and increase settlement.

2. The weight of the building increases non-linearly with increasing height, and thus the vertical load resting on the foundation can be considerable.

3. Lateral loads and moments due to wind are often cyclic. Therefore, the effect of cyclic vertical and lateral loading on the foundation system must be considered, because cyclic loading can weaken the capacity of the foundation and increase settlement.

4. We see that Tall buildings are often surrounded by low podium structures that are subjected to much lower loads. So, the differences between the high and low parts settlement must be checked.

## **SHEAR WALL**

A shear wall is vertical element of system which is designed to resist lateral forces due to wind and earthquake, which is one of the most efficient methods of maintaining the lateral stability of tall buildings. In practice, shear walls are generally provided in most of the commercial and residential buildings up to thirty storeys beyond which tubular structures are recommended. Shear walls may be provided in one plane or in both planes. The typical shear wall system with shear walls located in both the planes and subjected to the lateral loads.

The shear walls are expected to resist large lateral loads (due to earthquake or wind forces) that may strike "in-plane" and "out-of-plane" to the wall. The in-plane shear resistance of the shear wall can be calculated by subjecting the wall to the lateral loads.

Following are the basic terms that we are going to analyze:

**Storey drift:** Storey drift is the lateral displacement of floor relative to the floor below. Also, it is the difference of displacements between two consecutive storey's divided by the height of that story.

**Joint Displacement:** It is the displacement of the point on each storey with respect to the base storey or basement

**Base Shear:** It is an estimate of the maximum expected lateral force on the base of the structure due to seismic activity.

## **II. LITERATURE REVIEW:**

The extensive literature review was carried out by referring standard journals, reference books, I.S. codes and conference proceeding. The major work carried out by different researchers is summarized below.

**Yin Zhou and Ahsan Kareem**, in this paper, "Load Factors for Design Applications". Wind loads on structures caused by gusts have traditionally been addressed by the "wind load factor" (GLF) method in most major codes and standards. In the world The equivalent static wind load used in the design is equal to the average wind force multiplied by the GLF. Although the traditional GLF method can provide an accurate estimate of the shear response, it cannot provide a reliable estimate of any other response components. To overcome this shortcoming, this paper proposes a more realistic procedure for design loads.

**Wakchaure M.R., Gawali Sayali**, in this paper, the wind-wind efficiency factor method takes into account the dynamic characteristics of the structure, wind-structure interaction and then determines the wind loads as equivalent static loads. Wind loads are determined according to gust using the efficiency factor method. Critical structural blast loads are determined. After applying the wind loads calculated in the finite element software to the building models produced by ETAB 13.1.1v. Different designs are compared in various aspects such as slab displacements, ply deflections, ply displacements, column axial forces, etc. Based on the results, conclusions are made that show the effectiveness of different forms of the structure under the influence of wind loads.

**Mohammed Asim Ahmed, Moid Amir, Savita Komur, Vaijainath Halhalli**, this paper present movements occur in different terrain classes due to wind in different layers. The three models are analyzed with the ETABS 2015 package. The present works provide a good source of information on the variation of deflection with the change of model height and the percentage change of deflection of the same model in different terrain classes.

**Jadhav A. A, Dr. Kulkarni S. K. , Galatage A. A.** In this work, the main objective of the study is to determine the location of shear walls in a multi-story building. The earthquake load is directed to the 26th floor building in zone iii. The analysis is performed using Etabs software. Axial force, shear force, bending moment, layer displacement and time period are calculated and the location of the shear walls is determined.

### III. METHODOLOGY

Analysis of the different position of shear wall in building is done in following sequence.

1. First, we have to collect all relevant data regarding analysis of building such as seismic zone, terrain, soil type etc.
2. Then we have to do the study of shear wall and its behaviour.
3. Then preparation of G+18 model in E-Tabs.
4. After this we study different models related to our design.
5. Study the base reaction on each column.
6. Result obtained and their discussion.
7. Conclusion.

### IV. PROBLEM STATEMENT

In this project, a G+18-storey structure of a rectangular building and unsymmetrical building with 3 m floor to floor height have been analyzed Non-Linear Dynamic Analysis of Multi-storey R.C.C Buildings using Etabs software in zones III. The structure has been analysed for both static and dynamic wind and earthquake forces. The building has been studied for without shear wall, shear wall at corner and shear wall at outer centre position.

#### MODEL DESCRIPTION FOR ANALYSIS:

**Preliminary data required for Analysis:**

**Table 1: Parameters to be consider for rectangular geometry analysis**

| SR NO | Parameters               | Design Values         |
|-------|--------------------------|-----------------------|
| 1.    | No of stories            | G+18                  |
| 2.    | Base to plinth           | 1.5m                  |
| 3.    | Grade of concrete        | M30                   |
| 4.    | Grade of steel           | Fe 500                |
| 5.    | Floor to floor height    | 3m                    |
| 6.    | Total height of building | 58m                   |
| 7.    | Dead load                | 1.5 kn/m <sup>2</sup> |
| 8.    | Imposed load             | 4 kn/m <sup>2</sup>   |
| 9.    | Assumed city             | Nashik                |
| 10.   | Basic wind speed         | 39 m/s <sup>2</sup>   |
| 11.   | Terrain category         | Type 2                |
| 12.   | Size of column           | 500mm*500mm           |
| 13.   | Size of beam             | 300mm*500mm           |
| 14.   | Depth of slab            | 125mm                 |

#### Models:

There are six different types of models used for analysis:

1. Model 1: G+18 Rectangular building without shear wall
2. Model 2: G+18 Rectangular building with shear wall at corner position
3. Model 3: G+18 Rectangular building with shear wall at outer center position
4. Model 4: G+18 Unsymmetrical building without shear wall
5. Model 5: G+18 Unsymmetrical building with shear wall at outer center position
6. Model 6: G+18 Unsymmetrical building with shear wall at corner position

Following are the models for analysis:

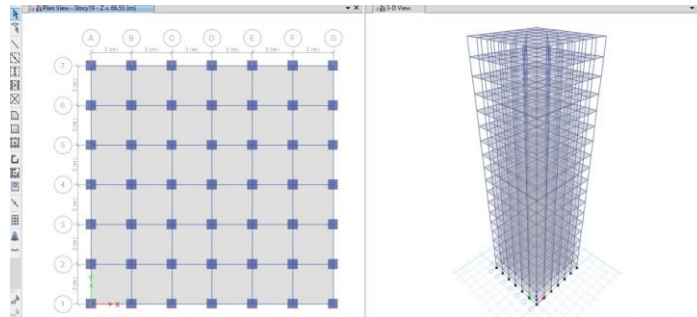


Fig 1. Rectangular building without shear wall

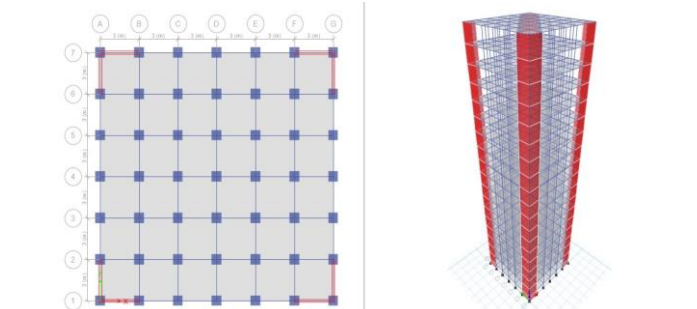


Fig 2. Rectangular building with shear wall at corner position

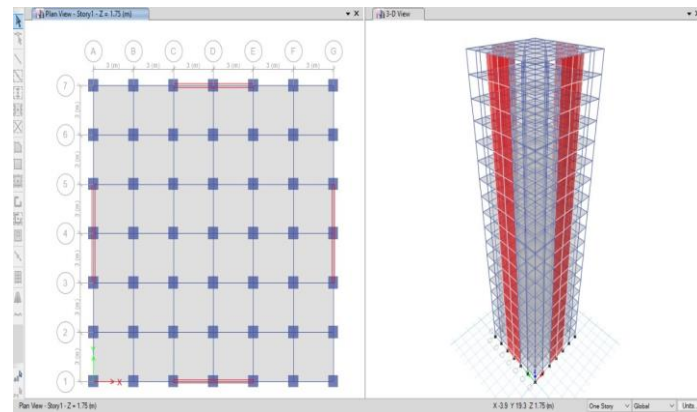


Fig 3. Rectangular building with shear wall at outer centre position

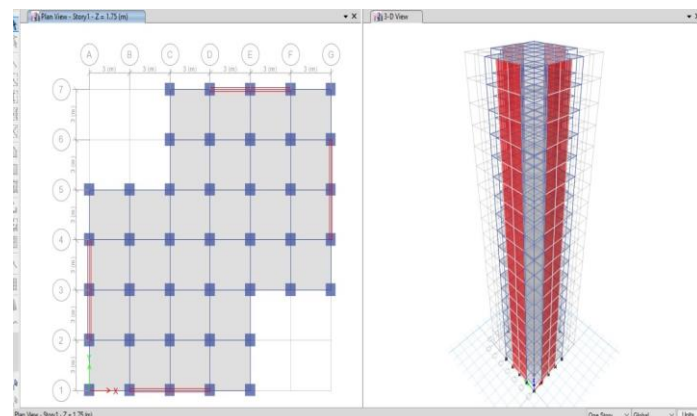


Fig 4. Unsymmetrical building with shear wall at outer centre position

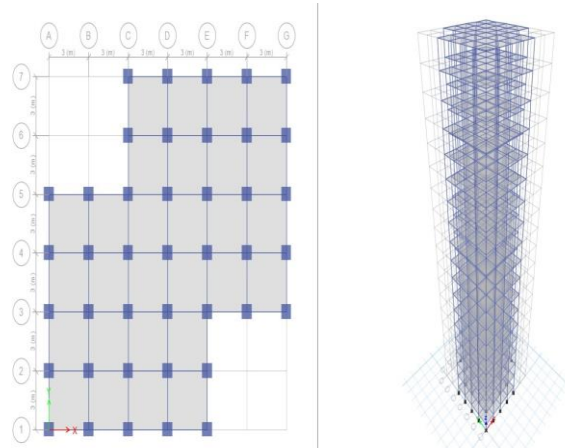


Fig 5. Unsymmetrical building without shear wall

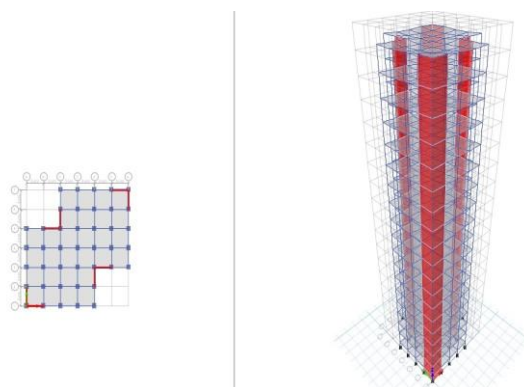


Fig 6. Unsymmetrical building with shear wall at corner position

### V. RESULT AND DISCUSSION

In this project, a G+18-storey structure of a rectangular building and unsymmetrical building with 3 m floor to floor height has been analysed Non-Linear Dynamic Analysis of Multi-storey R.C.C Buildings using Etabs software in zones III. The structure has been analysed for both static and dynamic wind and earthquake forces.

Results are given below:

#### RESULTS FOR RECTANGULAR BUILDING:

##### 1) Storey drift

| STOREY | without SW | SW at corner | SW at outer centre |
|--------|------------|--------------|--------------------|
| 18     | 1.285      | 2.134        | 2.055              |
| 17     | 1.704      | 2.312        | 2.223              |
| 16     | 2.13       | 2.522        | 2.411              |
| 15     | 2.477      | 2.702        | 2.558              |
| 14     | 2.747      | 2.861        | 2.691              |
| 13     | 2.97       | 2.999        | 2.807              |
| 12     | 3.164      | 3.101        | 2.879              |
| 11     | 3.338      | 3.199        | 2.947              |
| 10     | 3.492      | 3.28         | 2.999              |
| 9      | 3.631      | 3.34         | 3.033              |
| 8      | 3.756      | 3.375        | 3.046              |
| 7      | 3.865      | 3.377        | 3.034              |
| 6      | 3.956      | 3.336        | 2.984              |
| 5      | 4.035      | 3.232        | 2.882              |
| 4      | 4.1        | 3.041        | 2.705              |

|      |       |       |       |
|------|-------|-------|-------|
| 3    | 4.098 | 2.727 | 2.425 |
| 2    | 3.85  | 2.238 | 2.007 |
| 1    | 2.865 | 1.492 | 1.389 |
| G    | 0.962 | 0.546 | 0.542 |
| BASE | 0     | 0     | 0     |

**2) Joint displacement**

| STOREY | without SW | SW at corner | SW at outer centre |
|--------|------------|--------------|--------------------|
| 18     | 190.506    | 172.614      | 158.234            |
| 17     | 186.638    | 165.874      | 151.939            |
| 16     | 181.785    | 158.741      | 144.96             |
| 15     | 175.802    | 151.036      | 137.514            |
| 14     | 168.758    | 142.739      | 129.597            |
| 13     | 160.74     | 133.852      | 121.206            |
| 12     | 151.824    | 124.383      | 112.367            |
| 11     | 142.076    | 114.374      | 103.108            |
| 10     | 131.562    | 103.866      | 93.469             |
| 9      | 120.336    | 92.911       | 83.49              |
| 8      | 108.449    | 81.58        | 73.229             |
| 7      | 95.947     | 69.968       | 62.766             |
| 6      | 82.872     | 58.204       | 52.209             |
| 5      | 69.253     | 46.467       | 41.715             |
| 4      | 55.114     | 35           | 31.496             |
| 3      | 40.54      | 24.144       | 21.847             |
| 2      | 25.847     | 14.368       | 13.16              |
| 1      | 11.998     | 6.325        | 5.949              |
| G      | 1.683      | 0.956        | 0.949              |

**3)Base shear**

| STOREY | without SW | SW at corner | SW at outer centre |
|--------|------------|--------------|--------------------|
| 18     | 1800.7282  | 2838.9208    | 3033.1344          |
| 17     | 3902.2814  | 5810.1537    | 6302.6077          |
| 16     | 5473.5115  | 7732.7963    | 8498.2071          |
| 15     | 6551.1643  | 8966.665     | 9942.3165          |
| 14     | 7318.5726  | 9856.4101    | 10957.2771         |
| 13     | 7977.9282  | 10554.8005   | 11694.1765         |
| 12     | 8619.7062  | 11129.9169   | 12239.6095         |
| 11     | 9228.7953  | 11675.0271   | 12722.5025         |
| 10     | 9787.0953  | 12235.2137   | 13234.2724         |
| 9      | 10319.4464 | 12793.9317   | 13792.4995         |
| 8      | 10845.8288 | 13360.3078   | 14420.12           |
| 7      | 11341.6273 | 13992.0212   | 15176.104          |
| 6      | 11783.6045 | 14718.6013   | 16075.8448         |
| 5      | 12218.9638 | 15524.4002   | 17066.5708         |
| 4      | 12739.6303 | 16409.3093   | 18103.9349         |
| 3      | 13357.1085 | 17354.4511   | 19151.7528         |
| 2      | 13930.1871 | 18207.6427   | 20075.4245         |
| 1      | 14264.7449 | 18727.5393   | 20647.8514         |
| G      | 14308.9432 | 18810.3708   | 20743.6346         |

**RESULTS FOR UNSYMMETRICAL BUILDING**

**1) Storey drift**

| STOREY | without SW | SW at corner | SW at outer centre |
|--------|------------|--------------|--------------------|
| 18     | 0.927      | 1.283        | 1.541              |
| 17     | 1.198      | 1.372        | 1.635              |
| 16     | 1.473      | 1.467        | 1.751              |
| 15     | 1.696      | 1.553        | 1.84               |
| 14     | 1.867      | 1.626        | 1.91               |
| 13     | 2.006      | 1.685        | 1.962              |
| 12     | 2.126      | 1.726        | 1.978              |
| 11     | 2.232      | 1.761        | 2.002              |
| 10     | 2.325      | 1.786        | 2.015              |
| 9      | 2.407      | 1.803        | 2.016              |
| 8      | 2.48       | 1.808        | 2.005              |
| 7      | 2.542      | 1.8          | 1.978              |
| 6      | 2.593      | 1.77         | 1.928              |
| 5      | 2.636      | 1.711        | 1.845              |
| 4      | 2.67       | 1.608        | 1.715              |
| 3      | 2.66       | 1.441        | 1.522              |
| 2      | 2.49       | 1.182        | 1.244              |
| 1      | 1.843      | 0.788        | 0.849              |
| G      | 0.616      | 0.294        | 0.334              |

**2) Joint displacement.**

| STOREY | without SW | SW at corner | SW at middle |
|--------|------------|--------------|--------------|
| 18     | 126.296    | 93.708       | 106.662      |
| 17     | 123.477    | 89.893       | 102.026      |
| 16     | 120.008    | 85.689       | 96.823       |
| 15     | 115.825    | 81.219       | 91.316       |
| 14     | 110.971    | 76.482       | 85.556       |
| 13     | 105.505    | 71.479       | 79.545       |
| 12     | 99.478     | 66.219       | 73.292       |
| 11     | 92.937     | 60.741       | 66.893       |
| 10     | 85.922     | 55.052       | 60.324       |
| 9      | 78.471     | 49.171       | 53.607       |
| 8      | 70.616     | 43.129       | 46.778       |
| 7      | 62.388     | 36.966       | 39.887       |
| 6      | 53.813     | 30.742       | 33.002       |
| 5      | 44.909     | 24.544       | 26.221       |
| 4      | 35.689     | 18.491       | 19.679       |
| 3      | 26.206     | 12.759       | 13.56        |
| 2      | 16.67      | 7.596        | 8.108        |
| 1      | 7.713      | 3.35         | 3.639        |
| G      | 1.079      | 0.515        | 0.585        |
| BASE   | 0          | 0            | 0            |

**3) Base shear.**

| STOREY | without SW | SW at corner | SW at outer centre |
|--------|------------|--------------|--------------------|
| 18     | 965.1636   | 1455.7167    | 1684.6466          |
| 17     | 2092.2578  | 3025.1913    | 3560.7589          |
| 16     | 2922.9587  | 4038.8038    | 4835.4029          |
| 15     | 3479.739   | 4629.1753    | 5637.726           |
| 14     | 3866.3054  | 4991.4164    | 6147.3756          |
| 13     | 4196.6885  | 5276.1279    | 6497.3006          |
| 12     | 4521.478   | 5528.2626    | 6754.0001          |
| 11     | 4831.1743  | 5744.0763    | 6970.7092          |
| 10     | 5114.8595  | 5942.0949    | 7196.0582          |
| 9      | 5386.3771  | 6169.5037    | 7479.9297          |
| 8      | 5656.8552  | 6454.2393    | 7856.8432          |
| 7      | 5912.0907  | 6784.7431    | 8328.0583          |
| 6      | 6139.0898  | 7151.8221    | 8874.0614          |
| 5      | 6365.4837  | 7583.376     | 9480.2968          |
| 4      | 6644.3028  | 8105.3562    | 10131.5729         |
| 3      | 6981.9951  | 8669.2293    | 10773.8834         |
| 2      | 7297.9758  | 9147.8135    | 11300.8838         |
| 1      | 7482.6469  | 9419.9967    | 11604.9606         |
| G      | 7506.9193  | 9461.9952    | 11654.3203         |

**VI. CONCLUSION:**

In this study G+18-storey structure of a rectangular building and unsymmetrical building with shear wall provided at different location at corner, at outer centre and without shear wall analysed. Based on this study following conclusion can be drawn for the different positions of shear wall:

- The provision of shear wall can impact the seismic behavior of a rectangular structure to a large extent, and shear wall increases the strength and stiffness of the structure.
- When shear wall is provided at middle of outer, the joint displacement reduce by larger value for rectangular buildings.
- The utilization of shear wall significantly reduces the inter storey drift So shear wall at middle of outer centre should be optimum location.
- Base shear increases when shear wall is provided and it is maximum when shear wall is provided at middle of outer.
- The provision of shear wall enhances the lateral stability of the building specially for unsymmetrical structure.
- The joint displacement, inter storey drift reduces by greater value when shear wall provided at corner of structure in unsymmetric building.
- In view of unsymmetrical structure and eccentricity base shear increases at middle of the outer structure due to increases in mass and stiffness.
- By considering all the parameters shear wall at corner should be a better position due to immense decrement in interstorey drift and joint displacement.

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